

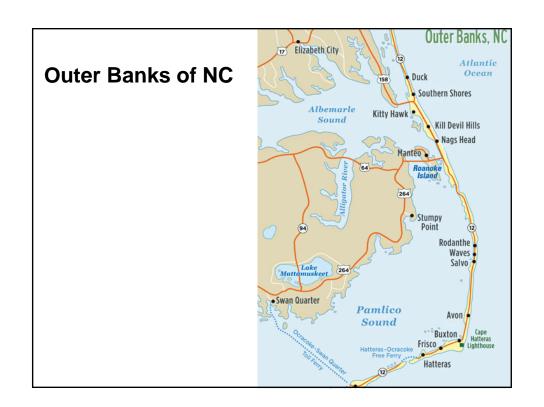
Verify Post-Scour Resistance of Jetted and Driven Pile Foundations

Mike Batten, PE Beth Howey, LG, PE



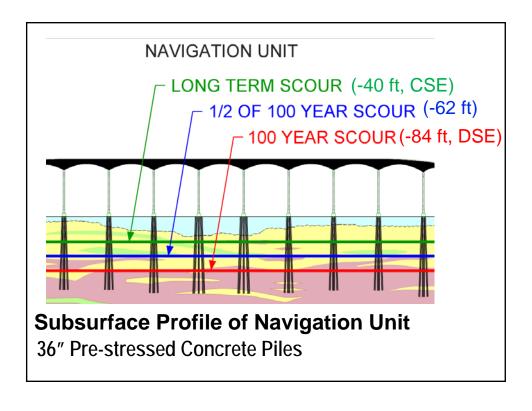
April 11, 2017

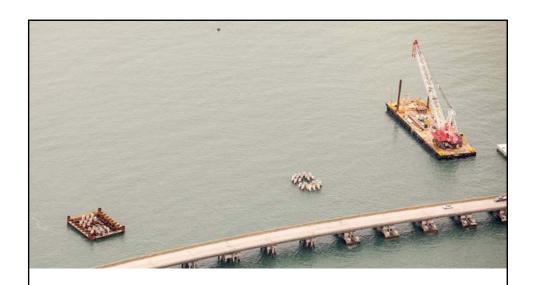










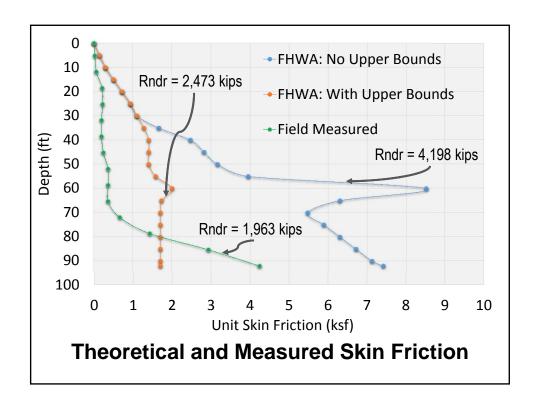


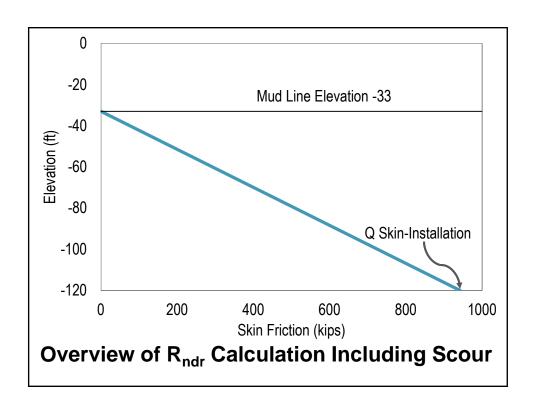
NAVIGATION UNIT PILES

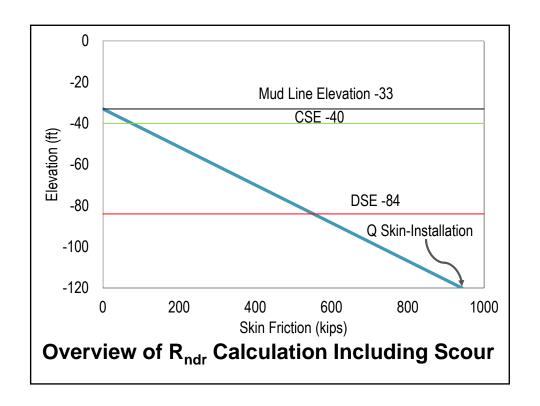
Required Nominal Driving Resistance (R_{ndr})

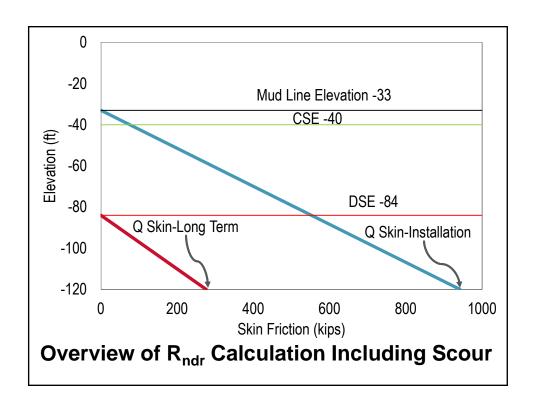
$$R_{ndr} = \frac{Factored \ Axial \ Load + Factored \ Dead \ Load}{\phi_{resistance}} \\ + \frac{Unfactored \ Scour \ Resistance \ (USR)}{\sigma_{resistance}} + \frac{Volume{1}}{\sigma_{resistance}}$$

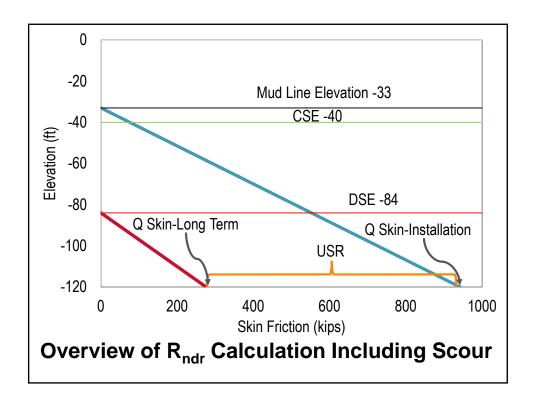
- AASHTO 10.7.3.3 states that scour resistance should not be factored so $\phi_{scour} = 1.0$
- $\blacksquare \, \varphi_{\text{resistance}}$ should be in accordance with AASHTO or NCDOT policy











$$f_s = \beta \times \sigma'_v$$

Where:

Fs = Unit skin friction

 β = Bjerrum Burland beta coefficient = $K_s tan \delta$

 σ'_{v} = Average effective stress along the pile shaft

K_s = Earth stress coefficient

 δ = Interface friction angle between pile and soil

Effective Stress Method to Determine Beta

Reference NHI Course Nos. 132021 and 132022 Design and Construction of Driven Pile Foundations

Characteristics of Beta

- β is primarily a function of:
 - oRelative density of soil
 - oPile soil/interface material
- ullet eta increases with densification from driving in pile groups
- ullet eta is NOT a function of effective or total stress!

Table 9-6 Approximate Range of Beta Coefficients (Fellenius, 1991)

| Soil Type | φ′ | β |
|-----------|---------|-------------|
| Clay | 25 – 30 | 0.23 - 0.40 |
| Silt | 28 – 34 | 0.27 – 0.50 |
| Sand | 32 – 40 | 0.30 - 0.60 |
| Gravel | 35 – 45 | 0.35 - 0.80 |

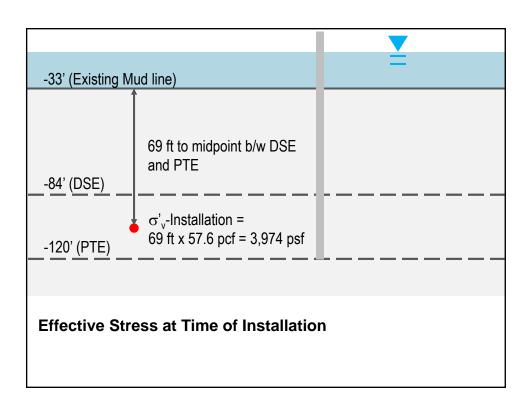
Range of Beta Coefficients

Reference NHI Course Nos. 132021 and 132022 Design and Construction of Driven Pile Foundations

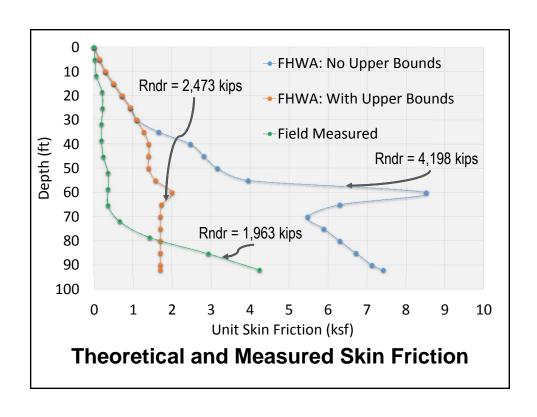
Method to Determine Beta

- Calculate effective overburden stress at the midpoint of installed pile between the Design Scour Elevation (DSE) and the Pile Tip Elevation (PTE) using ground surface at the time of pile installation.
- From CAPWAP data, determine the average measured unit skin friction for this zone.

•
$$q_s = \beta x \sigma'_v \rightarrow q_{skin-installation} = \beta x \sigma'_{v-installation}$$

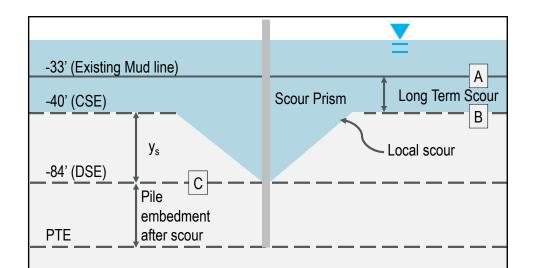


| | | | CAPW | AP SUMMARY | RESULTS | | | |
|-----------|-------------|---------|------------|------------|--------------------------|------------|---------|---------|
| Total CAP | WAP Capacit | y: 2399 | 9.9; along | Shaft | 920.0; at | Toe 1479.9 | kips | |
| Soil | Dist. | Depth | Ru | Force | Sum | Unit | Unit | Smith |
| Sgmnt | Below | Below | | in Pile | of | Resist. | Resist. | Damping |
| No. | Gages | Grade | | | Ru | (Depth) | (Area) | Factor |
| | ft | ft | kips | kips | kips | kips/ft | ksf | s/ft |
| | | | | 2399.9 | | | | |
| 1 | 40.1 | 5.1 | 1.0 | 2398.9 | 1.0 | 0.20 | 0.02 | 0.153 |
| 2 | 46.8 | 11.8 | 5.0 | 2393.9 | 6.0 | 0.75 | 0.06 | 0.153 |
| 3 | 53.5 | 18.5 | 17.0 | 2376.9 | 23.0 | 2.54 | 0.21 | 0.153 |
| 4 | 60.2 | 25.2 | 18.0 | 2358.9 | 41.0 | 2.69 | 0.22 | 0.153 |
| 5 | 66.8 | 31.8 | 15.0 | 2343.9 | 56.0 | 2.24 | 0.19 | 0.153 |
| 6 | 73.5 | 38.5 | 15.0 | 2328.9 | 71.0 | 2.24 | 0.19 | 0.153 |
| 7 | 80.2 | 45.2 | 19.0 | 2309.9 | 90.0 | 2.84 | 0.24 | 0.153 |
| SE 8 | 86.9 | 51.9 | 29.0 | 2280.9 | 119.0 | 4.34 | 0.36 | 0.153 |
| 9 | 93.6 | 58.6 | 29.0 | 2251.9 | 148.0 | 4.34 | 0.36 | 0.153 |
| 10 | 100.3 | 65.3 | 29.0 | 2222.9 | 177.0 | 4.34 | 0.36 | 0.153 |
| 11 | 106.9 | 71.9 | 53.0 | 2169.9 | 230.0 | 7.93 | 0.66 | 0.153 |
| 12 | 113.6 | 78.6 | 115.0 | 2054.9 | 345.0 | 17.20 | 1.43 | 0.153 |
| 13 | 120.3 | 85.3 | 235.0 | 1819.9 | 580.0 | 35.16 | 2.93 | 0.153 |
| 14 | 127.0 | 92.0 | 340.0 | 1479.9 | 920.0 | 50.87 | 4.24 | 0.153 |
| Avg. S | naft | | 65.7 | | sured Skin ce Below D | 10.00 | 0.83 | 0.153 |
| T | oe . | | 1479.9 | to Calcula | | OL | 164.43 | 0.106 |



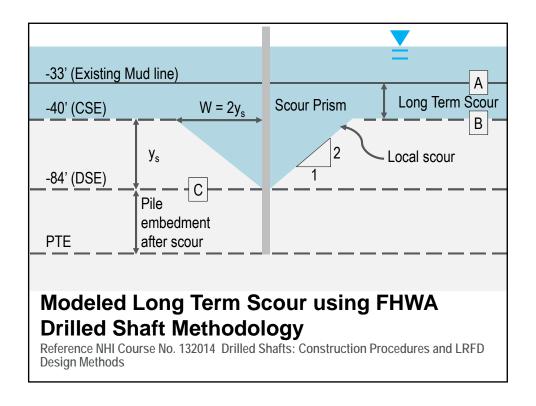


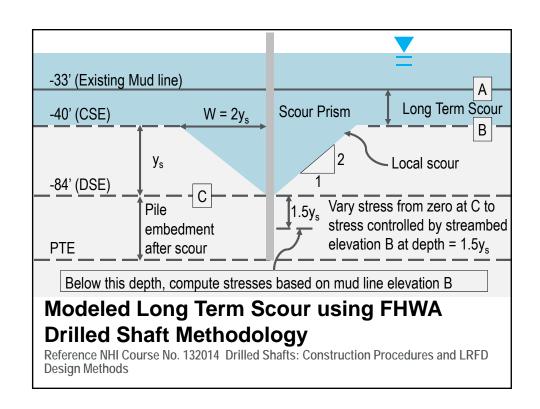
JETTING 36" SQUARE PILE

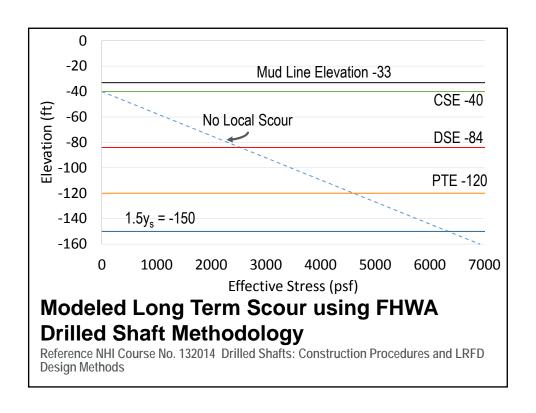


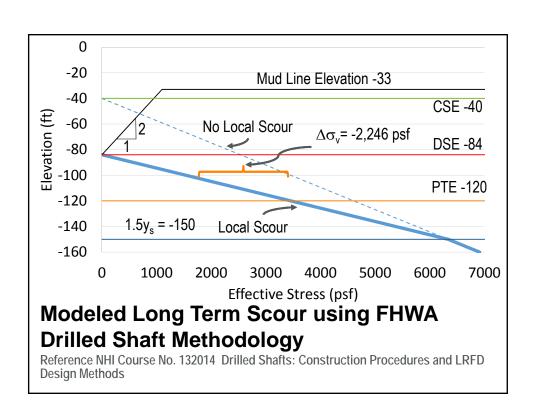
Modeled Long Term Scour using FHWA Drilled Shaft Methodology

Reference NHI Course No. 132014 Drilled Shafts: Construction Procedures and LRFD Design Methods









Calculate Long Term Skin Resistance

- Calculate the effective overburden stress after scour with procedure on previous slide with 2:1 scour hole from CSE to DSE.
- Using β calculated during installation, calculate long term skin friction.

•
$$q_s = \beta x \sigma'_v \rightarrow q_{skin-long term} = \beta x \sigma'_{v-scour}$$

 β is CONSTANT – it does not change after the scour event, only effective stress changes.

Calculate Unfactored Scour Resistance

 Unfactored Scour Resistance (USR) is the difference between long term post-scour skin friction and measured skin friction during pile installation.

Where:

q_{skin-long term} = Unit skin friction after scour event

A = Surface area of pile between DSE and PTE

Q_{skin-installation} = Skin friction resistance from CAPWAP



DRIVING 36" SQUARE PILE

